

FLOODPLAIN SEDIMENTATION AND SENSITIVITY: SUMMER 1993 FLOOD, UPPER MISSISSIPPI RIVER VALLEY

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ABSTRACT

Patterns of overbank sedimentation in the vicinity of, and far removed from, levee breaks that occurred in response to the >100 year, summer 1993 flood in the upper Mississippi River valley are elucidated. Two suites of overbank deposits were associated with the failure of artificial levees within a 70 km long study reach. Circumjacent sand deposits are a component of the levee break complex that develops in the immediate vicinity of a break site. As epitomized by the levee break complex at Sny Island, these features consist of an erosional, scoured and/or stripped zone, together with a horseshoe-shaped, depositional zone. At locales farther removed from the break site, the impact of flooding was exclusively depositional and was attributed to the settling of suspended sediment from the water column. The overall picture was one of modest scour at break sites and minimal suspended sediment deposition (<4 mm) at locales farther removed from the breach.

Downriver from the confluence with the Missouri River, suspended sediment deposition was of a similar magnitude to that observed within the study reach and levee break complexes exhibited a similar morphology, but scour at break sites was greatly enhanced and the excavated sand formed extensive deposits on the floodplain surface. The different erosional response was probably engendered by the higher sand content and reduced aggregate cohesion of the floodplain soils downriver from the confluence with the Missouri River. A qualitative comparison serves to highlight the influence that the resistance threshold may have on the sensitivity of floodplains bordering large low-gradient rivers to high magnitude floods.

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INTRODUCTION

Floodplain sedimentation occasioned by any given overbank flow is known to be characterized by substantial spatial variability (Walling *et al.*, 1992). Information about patterns of overbank deposition is required to facilitate understanding of floodplain development (Nanson and Croke, 1992), as well as to test models of sediment dispersal across floodplain surfaces and elucidate the fate of contaminated matter that is transferred to the floodplain environment (cf. James, 1985; Pizzuto, 1987; Howard, 1992; Bradley and Cox, 1986; Marron, 1989; Sahu *et al.*, 1994; Nicholas and Walling, 1995). Event-based field studies typically yield at-a-point information about the likely range and magnitude of overbank sedimentation that is derived from sediment traps or post-flood surveys (e.g. Grover, 1938; Kesel *et al.*, 1974; Brown, 1983; Gretener and Strömquist, 1987). Spatial variations in rates of overbank deposition averaged over a period of years have also been delineated through the use of environmental radionuclides, such as ¹³⁷Cs (Walling *et al.*, 1992). However, even on a flood-by-flood basis, little is known about the large-scale spatial variability of overbank sedimentation because limitations on sampling densities practicably confine field surveys to parcels of land that typically are of the order of a few square kilometres in area (cf. Asselman and Middelkoop, 1995; Simm, 1995).

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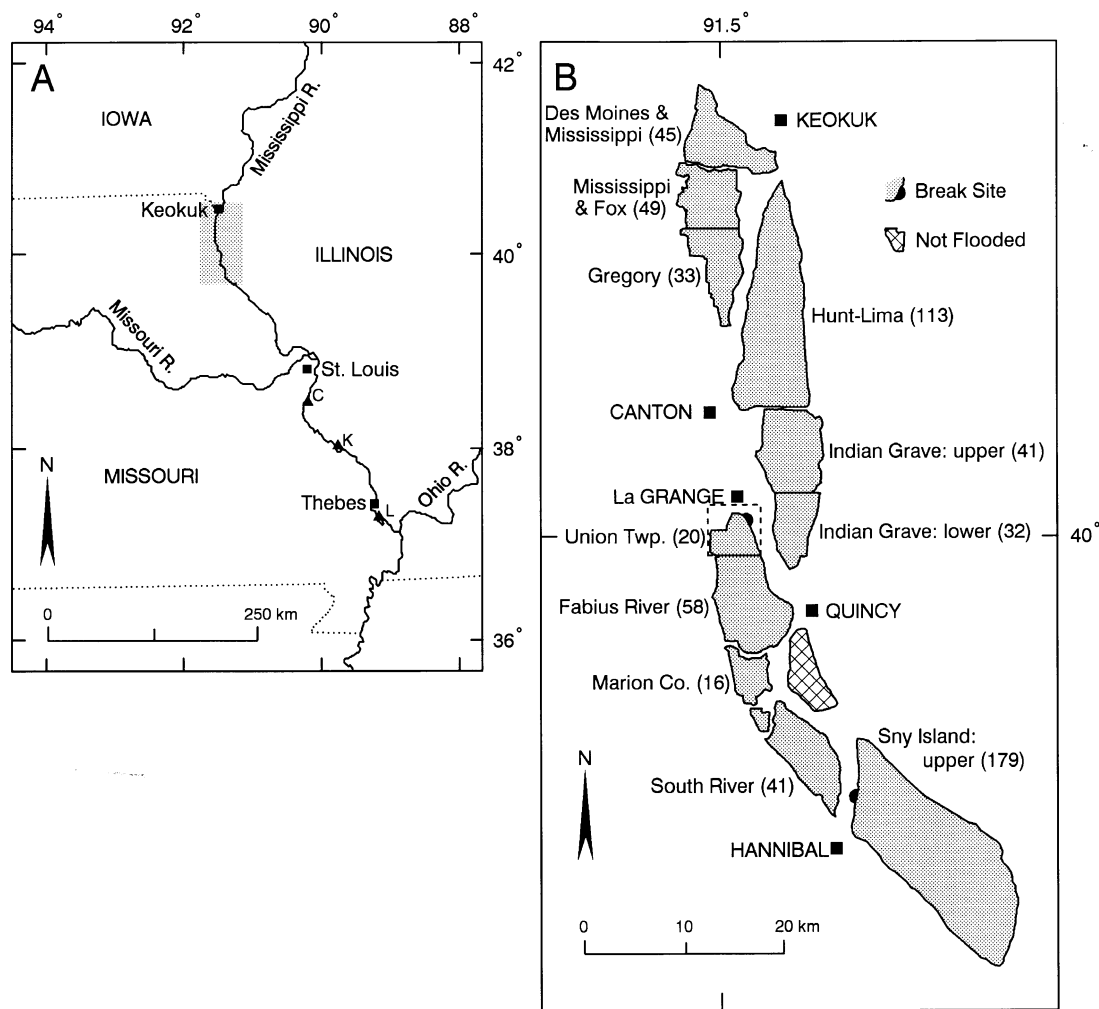


Figure 1. (A) Location of study reach and the sites of other levee break complexes referred to in the text. C=Columbia Levee, K=Kaskaskia Island Levee, and L=Len Small Levee. (B) Levee and drainage districts, principal towns and the location of break sites within the study reach referred to in the text. Figures in parentheses indicate the areal extent of flooding (km^2). The dashed box indicates the location of Figure 6.

The purpose of this paper is to elaborate on the pattern of overbank sedimentation associated with the point failure of artificial levees during the summer 1993 flood in the upper Mississippi River basin. These failures inundated clearly delineated, 16 to 179 km^2 sections of the floodplain (Figure 1). The point failure of an artificial levee mimics flood-basin inundation following the formation of crevasses in a natural levee. In this overbank environment, sediment transport and the resulting pattern of deposition are conditioned by water and sediment delivered from a discrete source (cf. Popov and Gavrin, 1970; Knight, 1975; O'Brien and Wells, 1986; Farrell, 1987). We draw on information about the nature, magnitude and spatial variability of the overbank deposits derived from post-flood field surveys and a calibrated TM image (Gomez *et al.*, 1995). Specifically we focus on: (i) the levee break complex at Sny Island and; (ii) the aggregate pattern of overbank deposition within the 20.2 km^2 Union Township Levee and Drainage District. We also examine the sensitivity of the riparian landscape in the upper Mississippi River valley to the summer 1993 flood by comparing the response of the floodplain in the study reach to that of the floodplain reaches further downriver, in the vicinity of breaks in the Kaskaskia Island, Columbia and Len Small levees (Chrzastowski *et al.*, 1994; Jacobson and Oberg, in press).

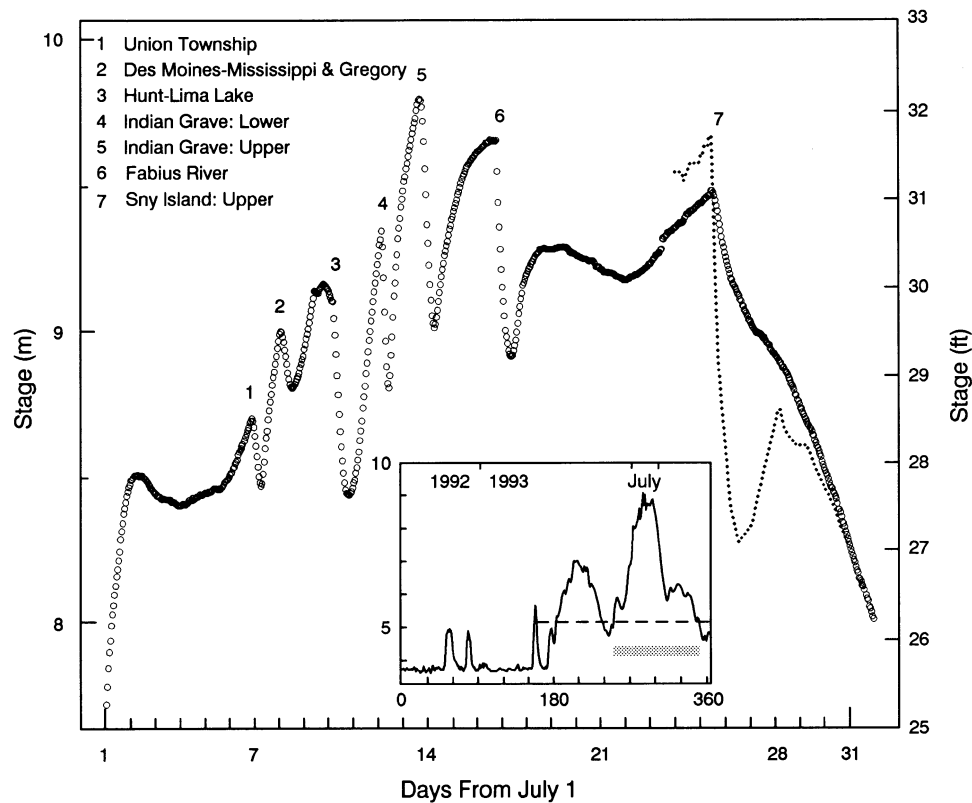


Figure 2. Hourly stage record at Quincy during July 1993, showing the effect of levee failures upriver (the reference level is provided by an arbitrary datum). The discontinuity in the stage record on 23 July was caused by a barge colliding with the gauge house. The stage record for Hannibal (dotted line) is superimposed to show the effect of the failure of the Sny Island Levee; the higher pre-failure stage is due to backwater effects upriver of the railway bridge at Hannibal. Inset shows the record of mean daily stage at Quincy for the 1992–3 water year. The dashed line indicates flood stage (5.18 m) and the stippled bar the duration of the summer 1993 flood.

SUMMER 1993 FLOODING AND THE STUDY REACH

The highest frequency of flooding in the upper Mississippi River basin as a whole is associated with the spring (April–June) snowmelt season, although the largest magnitude floods in the smaller tributary basins (of the order of a several hundred square kilometres in area) tend to occur during the summer months (June–August) in response to high-intensity excess rainfall (Knox, 1988). By contrast, the summer 1993 flood had its origins in an extended period of wet weather that began in the autumn of 1992 (Dowgiallo, 1994); it did not develop from a single catastrophic event. The pattern of wetter than average weather persisted over the upper midwestern United States for the first six months of 1993 and culminated with a series of intense storms in late June and July (Wahl *et al.*, 1993). The storms arose from the highly anomalous circulation evident throughout much of the northern hemisphere that locally allowed the easterly flowing jet stream and its associated surface front, which spawned the recurrent thunderstorm systems, to establish a quasi-stationary position over the northern portion of the Mississippi River basin (Dowgiallo, 1994). These climatic conditions were similar to those that prefaced the spring 1973 flood (Chin *et al.*, 1975; Wahl *et al.*, 1993; Parrett *et al.*, 1993).

Numerous gauging stations in the upper Mississippi River basin recorded record peak discharges during the summer of 1993, including gauges on the Mississippi River throughout the 300 km long reach between Keokuk, Iowa, and St Louis, Missouri, where the recurrence interval of the flood was >100 years (Parrett *et al.*, 1993). Our survey of the geomorphological effects of the 1993 flood focused on the 70 km long reach between Keokuk, Iowa, and Hannibal, Missouri (Figure 1). At Keokuk, the Mississippi River drains an area of 308 000 km². During the summer 1993 flood the peak discharge of 12 320 m³ s⁻¹ was about 20 per cent larger than the previous

maximum recorded (1973: $9735\text{ m}^3\text{ s}^{-1}$) and estimated (1851: $10195\text{ m}^3\text{ s}^{-1}$) peak discharges. The flood peak passed through the study reach between 10 and 16 July. At Quincy, Illinois, which is located in the central portion of the study reach, the river crested 4–6 m above flood stage (5–18 m) on 13 July. However, the stage did not begin to fall appreciably until 25 July and the river remained above flood stage for 101 consecutive days (Figure 2).

Within the study reach the river channel is about 1 km wide. The floodplain ranges from 8 to 12 km wide and is bordered by prominent bluffs. Its very subdued relief (typically <3 m) has been reinforced by agricultural activity. Upriver from the confluence with the Missouri River, the sediment load of the Mississippi River consists predominantly of silt- and clay-sized material, and the floodplain soils are very cohesive (cf. Bushue, 1979). Prior to and throughout the historic period, the position of the river has remained stable (Van Nest, 1990), but navigation and flood protection works undertaken over the past 150 years have constricted the main channel, decoupled the river from its floodplain, and locally enhanced flood stages (Belt, 1975; Chin *et al.*, 1975; Chen and Simons, 1986). However, long-term trends in flooding in the upper Mississippi River basin are strongly correlated with a coincident increase in average annual precipitation (Knapp, 1994).

During the summer 1993 flood, 1082 of the 1576 federal and non-federal levees within the Missouri and upper Mississippi river basins failed, flooding some $93\,000\text{ km}^2$ of land. Within the study reach, all or part of 11 levee and drainage districts, accounting for 630 km^2 of protected floodplain (Figure 1), and $>200\text{ km}^2$ of unleveed floodplain, were inundated. Along the upper Mississippi River the crown elevation of main-channel levees corresponds to a minimum 50 year flood design with 0–61 m freeboard, although the design criteria for levees protecting urban and industrial areas are more rigorous than those for agricultural levees. For agricultural levees within the study reach, the design flood typically corresponds to a river stage of c. 8 m (cf. Figure 2). The levees typically are linear features of the order of 5 to 7 m high, with 1:3 to 1:5 river and floodplain side slopes, and a c. 3 m wide crown. They are 40 to 50 m wide at the base and are constructed from locally available material dredged from the river or excavated from the floodplain. During the summer of 1993, the crown of vulnerable sections of many levees was temporarily raised by steepening the floodplain side slope profile. At Sny Island, the crown elevation at several locations was increased by vertical boards backed by sandbags, that were draped with plastic sheeting to protect against wave action (cf. Stewart, 1993).

Levee failures within the study reach occurred primarily as a consequence of overtopping linked with wave-induced surface erosion rather than structural failure, but a number of saturation-induced mass failures were precipitated by the high flood stages that persisted for much of July. In particular, side slope seepage and underseepage were recurrent problems along the Sny Island levee, and were probably responsible for its eventual failure (Chrzastowski *et al.*, 1994). Because the levee systems compartmentalized the floodplain, the failure of any given main-channel levee was associated with flooding within a well-defined area, the integrity of which was preserved because the crowns of the encircling levees projected above the impounded floodwater (cf. Gomez *et al.*, 1995). Depending on the size of the area involved, water levels in the river and on the floodplain were equalized over a period of 7 to 15 h (Figure 2). Thereafter the transfer of water and sediment across the channel–floodplain boundary was curtailed and the breach deactivated because there was no outlet for the floodwater.

FIELD SURVEY AND REMOTE SENSING

We made a preliminary aerial reconnaissance of the study reach in November 1993. On the basis of this appraisal we undertook a detailed field survey of three sections of leveed floodplain and a 15 km long section of unleveed floodplain in the vicinity of Canton, Missouri, in the winter of 1993–1994. The levee break complex at Sny Island levee and the Union Township Levee and Drainage District were selected for detailed study because the respective failures occurred following and in advance of the passage of the flood peak through the study reach (Figure 2). Both levee break complexes displayed a similar morphology, although the Sny Island complex was much larger than the other levee break complexes in the study reach. The fields in the vicinity of the break sites were planted with row crops (soya beans or maize).

At each break site we profiled and determined the depth of the circumjacent sand deposits by probing and digging shallow pits along a series of some five to ten transects. High-water marks on trees and buildings within

view of a break site were also surveyed. The summer 1993 flood deposits typically were separated from the pre-existing soil by crop residue, and could also be differentiated on the basis of colour. We sampled and measured the thickness of the overbank (sand and silt) deposits in soil pits dug at c. 100 m intervals along a 1 km long profile originating at the break site, and recorded the thickness of sediment present at numerous (>100) other randomly selected locations on the floodplain. No corrections for moisture content were made because the flood deposits had dried out by the time our field surveys were undertaken. Particle-size distributions were determined by dry sieving and/or laser granulometry.

Remotely sensed data have long been used to provide a synoptic perspective on flooding (e.g. Rango and Salomonson, 1974), and spectral reflectance affords the potential for deriving estimates of suspended sediment concentration in the surface water (e.g. Ritchie and Cooper, 1988; Stumpf and Pennock, 1989; Mertes *et al.*, 1993). A Landsat 5 Thematic Mapper (TM) image (path 25 row 32) was analyzed to derive estimates of near-surface suspended sediment concentration in the floodwater (Gomez *et al.*, 1995). The image was acquired prior to the failure of the Sny Island levee, on the morning of 25 July, one to two weeks after most of the levees in the study reach failed. Nevertheless, the overall pattern of sedimentation was preserved by virtue of the low settling velocity of the clay particles in suspension (cf. Mehta *et al.*, 1989) and because, once a break was deactivated, the continuous transfer of water and sediment across the channel–floodplain boundary was hindered by the crowns of levees which projected above the floodwater. Gomez *et al.*, (1995) noted that the pattern of suspended sediment transport across the floodplain was dominated either by a sediment plume originating at the break site, or a large-scale eddy that probably reflected the wind-driven circulation pattern that developed within the shallow, ponded floodwater (cf. Liggett and Hadjitheodorou, 1968).

Mertes *et al.* (1993) showed that after nominal calibration to water-surface reflectance, near-surface suspended sediment concentrations could be estimated for each 30×30 m pixel using linear spectral mixture analysis. Reflectance spectral end-members were based on data for a variety of sediment/water mixtures, and the end-member fractions were related to absolute sediment concentrations using a non-linear calibration curve (Mertes *et al.*, 1993; Mertes, 1994). Although the exact relation between surface and depth-integrated concentrations depends on local boundary shear stress, water depth and sediment size (McLean, 1992), near-surface suspended sediment concentrations approximate depth-average concentrations in shallow (<10 m deep) water (Ongley *et al.*, 1990; Mertes *et al.*, 1993). It was also assumed that all of the sediment in the water column settled out and the post-flood peak slack-water near-surface sediment concentrations could be used to characterize the loading during inundation. In such circumstances, if the depth of the water column is known, and in the absence of the continuous transfer of water and sediment onto the floodplain, the surface concentrations in the floodwater convert directly to an estimate of sediment thickness. The spatial distribution of overbank deposits within the 20 km² Union Township Levee and Drainage District was mapped on this basis.

SNY ISLAND LEVEE BREAK COMPLEX

The Sny Island levee break complex covered an area of about 2 km² in the immediate vicinity of the break site (Figure 3). It comprised a localized zone of concentrated scour, a more extensive zone of superficial stripping, and a circumjacent depositional zone. The levee borders McDonald Chute and is separated from the main channel by McDonald Island at low flows, but at the time the levee failed the depth of flow over the island was about 3 m. Although the final width of the break was about 1000 m, significant scour occurred only in the vicinity of the initial failure. The principal scour hole was of the order of 125 m long, 20 m wide and <2 m deep, and assumed the generalized form of the inverted frustum of an elliptical cone, with the deepest portion of the scour hole located on the floodplain side of the levee, downstream of the vena contracta (cf. Das, 1973). The volume of the scour hole was about 4000 m³. On the river side of the levee, scour upstream of the vena contracta directly linked the break site with the chute. Several smaller, secondary scour holes apparently developed on the opposing side of the breach as it widened, so that a pothole scour pattern was evidenced on the floodplain over a distance of about 300 m within the general vicinity of the initial break.

The 0.1 to 0.5 m deep spur-and-furrow topography of the stripped zone was the most prominent erosional feature associated with the levee break at Sny Island. Stripped zones characteristically develop in the vicinity of levee breaks (cf. Bhowmik, 1994; Chrzastowski *et al.*, 1994; Jacobson and Oberg, in press). They extend



Figure 3. Levee break complex at Sny Island. The length of the section of levee shown in the photograph is about 2.25 km. P=pothole scour in the vicinity of the initial failure, S=stripped zone, R=sand rim, and Ss=sand sheet. A–A' indicates the location of the transect depicted in Figure 5.

beyond the scour zone and, if a break widens, supplant localized scour as flow through the breach becomes more diffuse. The pattern of spur-and-furrow topography in the stripped zone of the Sny Island levee break complex primarily reflected the orientation of old plough furrows (running perpendicular to the levee), which probably helped concentrate the floodwater and initially accentuated erosion. However, flume experiments have demonstrated that analogous erosional bed features can also develop on initially plane, cohesive surfaces (Partheniades, 1965; Allen, 1969).

Two-dimensional model simulations of flooding in impounded areas following the point failure of artificial levees (e.g. Memos *et al.*, 1983; Popovska, 1989), and real-time video coverage of the failure (on 16 July) of the Fabius River levee provided an insight into the manner in which the floodwater spread across the floodplain. The hydrogeomorphological characteristics of flooding in the region near to the break, where the flow is dominated by inertial effects, differ from those in regions farther removed from the break site, where the floodplain topography and soil properties exert a fundamental control on the flow. When a levee fails, water is released through the breach in the form of a jet. The accelerating flow promotes scour in the vicinity of the break, that typically widens with time. A quasi-stable flow depth that is slightly less than the level of water in the channel is maintained at the levee. Away from the break site, the flow depth initially decreases with increasing distance across the floodplain. Near the wave-front the flow may locally become supercritical in the early stages of flooding. Reversion to a subcritical condition is marked by a hydraulic jump. On a plane surface the wave-front assumes an approximately semi-circular form, but its exact configuration and position are determined by the local floodplain topography and soil properties. Frictional energy losses help to increase the flow depth behind the wave-front as it spreads out across the floodplain. Once the wave-front reaches the floodplain boundaries the water depth increases in a more uniform manner. Depth continues to increase until the pressure gradient across the channel–floodplain boundary approaches zero and the breach is deactivated.

Little bed material encroached onto the floodplain through the breaks; overbank sedimentation is related to bank height (Harvey and Schumm, 1994). In the case of the break sites we examined, the transfer of sediment from the river onto the floodplain was inhibited by nearly vertical, 1 to 2 m high banks, as well as by the general

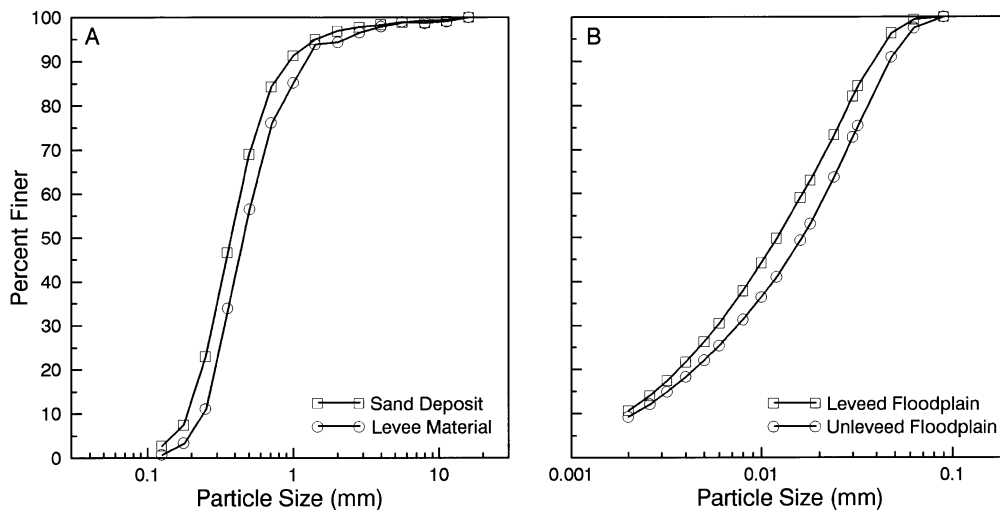


Figure 4. (A) Composite particle size distribution of the sand deposits within the levee break complex at Sny Island. The particle size distribution of a sample of levee sand is shown for comparison. (B) Composite particle size distribution of overbank deposits within the Union Township Levee and Drainage District. A composite curve for silt deposits found on unleveed sections of the floodplain adjacent to the main channel is shown for comparison.

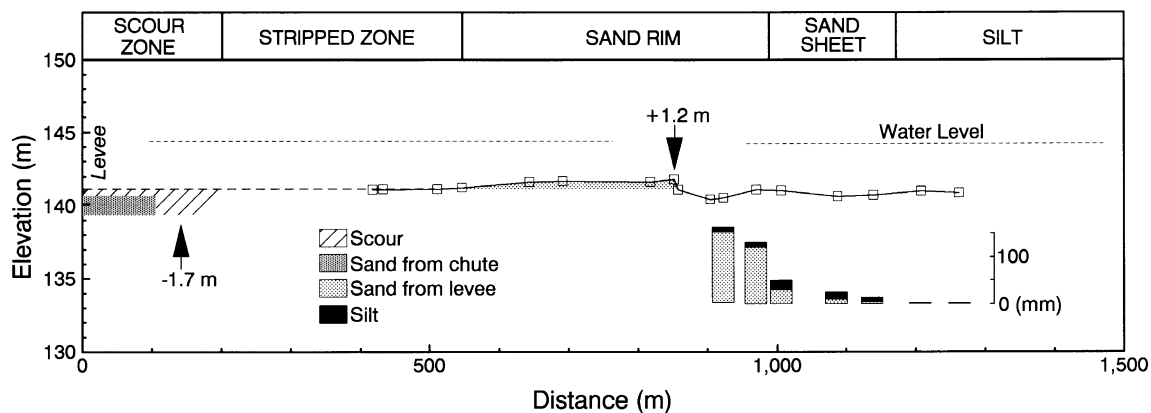


Figure 5. Transect across the Sny Island levee break complex. See Figure 3 for location.

inability of scour to initiate a comprehensive link between the channel and the floodplain. In the one case (at Sny Island) where a direct link was established, the bulk of the sand that was flushed from the McDonald Chute accumulated in and partially plugged the scour hole. The circumjacent sand deposits beyond the stripped zone thus essentially consisted of coarse sand derived exclusively from the break site that was smeared over the floodplain surface as the flood wave advanced across it. On the basis of the depth of sand recorded along our transects, we estimated that the volume of these deposits was of the order of $195\,000\text{ m}^3$. This figure compares favourably with the $190\,000\text{ m}^3$ of sand that the US Army Corps of Engineers used to repair the break. A composite size distribution for the circumjacent sand deposits is given in Figure 4A. These data also suggest that the sand deposit was derived from the levee material.

The configuration, depth and extent of sand deposition was influenced by the pattern of flow in the vicinity of the break site, and by the manner in which the breach widened. Unlike a crevasse splay deposit, which is a natural analogue for a levee break complex and is nourished by sediment that is ramped continuously onto the floodplain surface, the supply of material from a break is finite and the circumjacent sand deposits

characteristically take the form of a horseshoe-shaped sand rim that encircles the stripped zone (Figure 3). At Sny Island, the sand rim had a gently ramped backslope, several hundred metres long, that terminated in a well-defined slip face (Figure 5). Sand thicknesses of 0.15 to 1.3 m were typically observed at the crest, the location of which was probably constrained in the period immediately following failure by the position of the hydraulic jump that marked the transition to rapidly decelerating, divergent flow on the floodplain. The traction carpet in the zone of divergent flow is manifest by a splay-like sand sheet or discontinuous sand streaks that extend out across the floodplain beyond the leading edge of the sand rim (Figure 3). Where the sand sheet was not present, there was an abrupt transition between the leading edge of the sand rim and the surrounding, undisturbed floodplain surface. In other locations, although the transition appeared abrupt, the sand sheet was draped with silt which had settled from suspension (Figure 3). However, there were no obvious horizontal laminations within this silt unit or within any of the other silt deposits we scrutinized, which were all massively bedded (cf. Singh, 1972; Ray, 1976).

Features that exhibited essentially the same configuration (but which were neither as large nor as well preserved) as the levee break complex at Sny Island were associated with all of the levee breaks in the study reach (cf. Figure 5). They exemplify the erosional effects of flooding and the suite of sediments present in the region near to a break. At locales farther removed from the break site, the impact of flooding was entirely depositional, and was primarily a function of the settling of suspended sediment from the water column after the break had been deactivated. A silt deposit, a few millimetres thick, draped the leading edge of the sand rim and the sand sheet that encircled the Sny Island break site (Figure 3), and a barely detectable veneer of silt was present at most surveyed locations on the floodplain that were far removed (>1.5 km) from the break site.

SUSPENDED SEDIMENT DEPOSITS AT UNION TOWNSHIP

The break in the Union Township levee was <100 m wide and the associated levee break complex covered an area of some 0.01 km². As estimated from the calibrated TM image, the magnitude and pattern of suspended sediment deposition resulting from the failure of the levee on 7 July are shown in Figure 6. For each of the four zones delimited in Figure 6, the magnitude (though not the extent) of the fine sediment deposition was corroborated by the point-specific field survey we conducted before the image was processed. The size distribution of the deposits is characterized by the composite curve presented in Figure 4B. The depth of sediment was greatest in the region immediately beyond the break site, where the impact of scour and stripping on the cohesive floodplain soils was most pronounced and towards which the main body of flow from the break site was presumably directed. Effects due to overtopping of the levee at other sites can also be discerned in Figure 6. On natural floodplains the decrease in the depth of overbank deposits with increasing distance from a river is well known, and previous field surveys on leveed floodplains have revealed that the depth of overbank deposits also decreases with increasing distance from a break site (e.g. Grover, 1938). Such observations are also consistent with theory (e.g. Bursik, 1995). However, this is the first time that the aggregate pattern of sediment deposition has been revealed. The amount of deposition is typical of the other (leveed and unleveed) locales we investigated, and our field survey suggests that it is also typical of the study reach as a whole. Commensurate with the timing of the summer 1993 flood, the small amount (typically a few millimetres) of overbank suspended sediment deposition that characterized this >100 year event was linked with low sediment concentrations in the main channel rather than the isolation of the floodplain from the main channel by levees (Gomez *et al.*, 1995).

FLOODPLAIN SENSITIVITY

The preceding case studies typify the erosional and depositional impact that the summer 1993 flood had on the study reach. Within the study reach, cohesive floodplain soils helped limit scour at break sites and there was no substantive channel change, suggesting that the resistance of the riparian landscape to erosion generally exceeded the erosive power of the summer 1993 flood. There is a correlation between the rate of energy dissipation of flowing water and hydraulic erodibility (Annandale, 1995), and the effect that large floods have on river and floodplain morphology has long been recognized (Melton, 1936). Resistance to erosion is

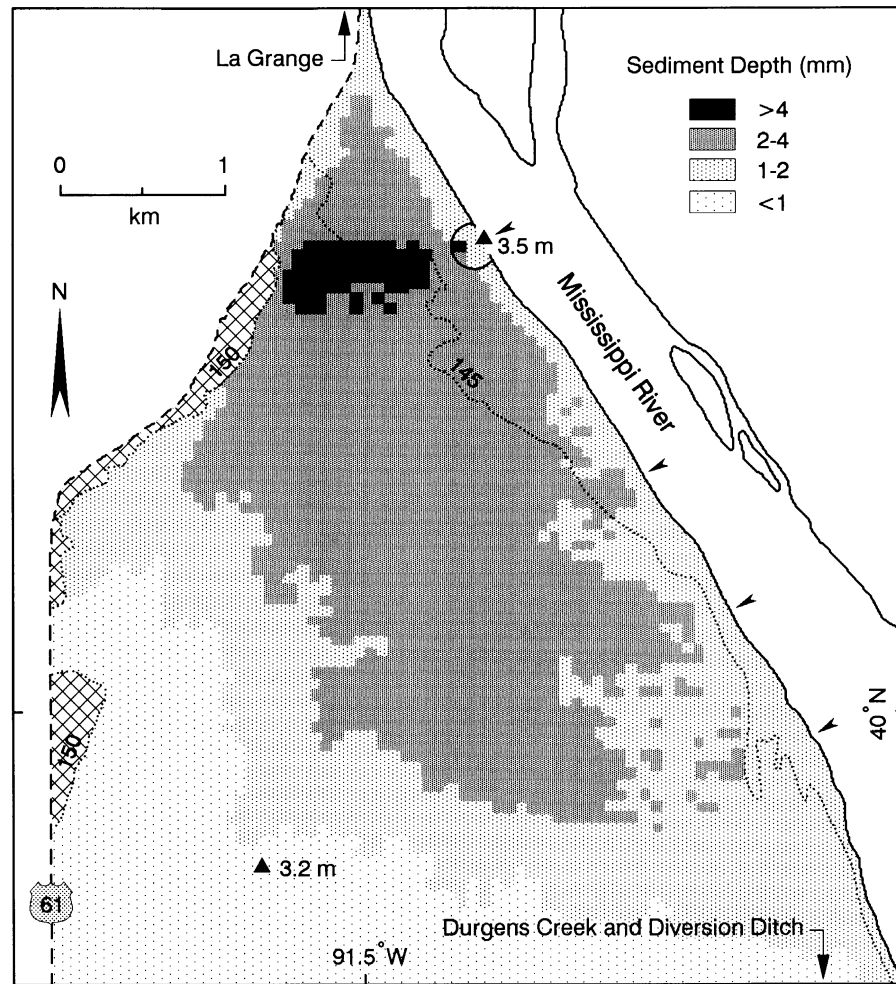


Figure 6. Spatial variation of the depth of overbank deposits within the Union Township Levee and Drainage District, south of La Grange, Missouri, derived and mapped from a calibrated TM image acquired on 25 July 1993 (Gomez *et al.*, 1995). See text for explanation. Land above 150 m in elevation (cross-hatched) and to the west of State Highway 61 was not flooded. The extent of the levee break complex is indicated by the semi-circle that surrounds the break site (arrowed). Other locations where overtopping of the main channel levee occurred are also arrowed. Figures adjacent to the solid triangles indicate the mean depth of water on the floodplain. See Figure 1B for location.

acknowledged to be a major factor determining the riparian landscape's response to large, rare floods (Bull, 1979; Brunsden, 1993), and Costa and O'Connor (1995) have commented on the dissimilar impact on channels and floodplains that floods of a similar magnitude and frequency often evince. Such comparisons are usually effected by considering the impact of different flood events on different river systems or, occasionally, the impact of different floods on the same river system (cf. Huckleberry, 1994). The summer 1993 flood afforded an opportunity to contrast the response of different sections of the Mississippi River floodplain to a single high-magnitude event.

The Missouri River transports a higher proportion of sand than the Mississippi River, and downriver from the confluence with the Missouri River the floodplain soils contain a correspondingly higher proportion of sand (Parks and Fehrenbacher, 1968; Higgins, 1987; Miles, 1988). Typically, although a shallow (<1 m deep) cohesive top-stratum characteristically is present, these soils are less cohesive than the soils in the study reach and the position of the channel is known to have changed frequently throughout the Holocene, as well as within

the historic period, primarily in response to cutoff development (cf. Fisk, 1944; Chrzastowski *et al.*, 1994; Jacobson and Oberg, in press). Record stages were recorded as far downriver as Thebes, Illinois, and the flood was equally protracted, although downriver from St Louis the recurrence interval of the 1993 flood was substantially less than 100 years (Parrett *et al.*, 1993; Bhowmik, 1994). Low flows in the Ohio River and an increase in channel capacity were responsible for the additional reduction in the magnitude of the summer 1993 flood that was experienced downriver from Cairo, Illinois (Dowgiallo, 1994). Under normal flow conditions two-thirds of the flow downriver from Cairo is contributed from the Ohio River.

Chrzastowski *et al.* (1994) and Jacobson and Oberg (in press) have described the levee break complexes that were associated with the failure of the Kaskaskia Island and Columbia levees, and the Len Small levee in southwest Illinois (Figure 1). The pattern of floodplain landuse in these areas was similar to that in the study reach. In as much as they were characterized by erosion and depositional patterns that embodied distinct scoured, stripped and depositional zones, their morphology was similar to that of levee break complexes in the study reach (cf. Jacobson and Oberg, in press). Low sediment concentrations in the main channel also reduced the transfer of suspended sediment to the floodplain and, where it was recognized, suspended sediment deposition was of a similar magnitude to that observed in the Union Township Levee and Drainage District.

The width of levee breaks in the reach downriver from St Louis typically was similar to that observed in the study reach but, by contrast, large, deep scour holes developed before the breach was deactivated, and thick, extensive sand deposits were left on the floodplain in the vicinity of and beyond break sites. In the case of the Kaskaskia Island levee break, the maximum depth of scour across the levee axis at the vena contracta was >15.25 m, and the total volume of the scour hole was $>800\,000\text{ m}^3$. At Columbia, the circumjacent sand deposit, that was mapped to a minimum thickness of 0.15 m (maximum thickness 2.44 m), covered an area of about 3 km^2 and extended up to 2.4 km beyond the break site. Both breaks were of the order of 150 m wide, and the scour failed to connect the break sites directly to the main channel. Thus the bulk of the sand deposited on the floodplain was derived from the scour holes (Chrzastowski *et al.* 1994).

Oberg and Jacobson (1994) characterized the continuously active break and the associated scour in the Miller City Levee and Drainage District as an incipient meander cutoff. As much as $8100\text{ m}^3\text{ s}^{-1}$ of water flowed through the breach, bypassing a 31 km long, high-amplitude meander bend before re-entering the main channel. The scour connected the break site directly to the main channel and the flow excavated an irregular channel, 2.2 km long and up to 25 m deep, across the floodplain, which was able to accommodate *c.* 25 per cent of the discharge of the Mississippi River. This channel facilitated the transfer of sediment from the river to the floodplain in the same manner as a natural crevasse. In excess of 80 km^2 of floodplain land were inundated by the failure of the Len Small levee (Jacobson and Oberg, in press). Sand deposits were estimated to cover an area of 75 km^2 and some 7.6 km^2 were subject to scour or stripping. Including the stripped zone, the total volume of scour was about $5.7 \times 10^6\text{ m}^3$. The total volume of the sand deposits was of the order of $13.9 \times 10^6\text{ m}^3$. Thus, transport of sediment from the main channel through the breach resulted in the net deposition of $>8.2 \times 10^6\text{ m}^3$ of sand on the floodplain, a figure that represents 22 to 36 per cent of the total sediment load of the Mississippi River at Thebes, Illinois, during the summer 1993 flood (Jacobson and Oberg, in press; Holmes, 1996).

In the study reach and elsewhere (cf. Jacobson and Oberg, in press), the style of flooding and the characteristic morphology of the levee break complexes were, in large part, conditioned by the long duration of the summer 1993 flood and the artificially enhanced hydraulic head that the levees themselves promoted. Whether or not overbank flooding of the magnitude experienced during the summer of 1993 would have produced a comparable response in the absence of the artificial levees is open to debate. The catastrophic disruption to the floodplain that occurred following the failure of the Len Small levee was occasioned by the development of active (channelized) flow across the floodplain in combination with the formation of a downriver outlet for the floodwater (Oberg and Jacobson, 1994). Nevertheless, in reaches downriver from the confluence with the Missouri River the overall picture was one of greatly enhanced scour at break sites coupled with extensive sand deposition. The strikingly different response was probably engendered by the higher sand content and reduced aggregate cohesion of the floodplain soils downriver from the confluence with the Missouri River.

There is a tendency to depict high-magnitude floods on large, low-gradient rivers, such as the Mississippi River, as generating a large amount of total energy while accomplishing minimal geomorphological change

because peak energy expenditure is considered too low to exceed the potential resistance threshold of the channel and floodplain (Costa and O'Conner, 1995). The results of our study ostensibly lend support to this notion. However, although confounded by a four-fold increase in stream power (the peak stream power at Keokuk and Thebes was 23 and 110 W m⁻², respectively), we suggest that the contrast in the magnitude of erosion and deposition associated with levee break complexes upriver and downriver from the confluence with the Missouri River primarily affords evidence of the dissimilar response that sections of floodplain with different resistance thresholds may have to a single flood event. At-a-station stream power is controlled by normal downstream changes in channel and floodplain properties (e.g. the shape of the longitudinal profile) and by local controls (e.g. valley constrictions). Thus a flood of a given flow frequency may generate a non-systematic downstream pattern of stream power. Specific stream power also has varying frequencies of occurrence within a drainage basin (Magilligan, 1992).

The flood characteristics (e.g. the peak flow and flow volume) which determine the amount of energy available for geomorphic work may be characterized by unique values and it is tempting to characterize the sensitivity of a floodplain to erosion in similar terms. For example, percentage (silt and) clay is correlated with the critical tractive force required to initiate motion of cohesive soil which contains a sand component (Dunn, 1959; Smerdon and Beasley, 1961), bank strength and resistance to lateral erosion are related to textural characteristics (Hicken and Nanson, 1984), and there is also a posited link between the sediment transport regime of a river and its sedimentary record (Pizzuto, 1985). However, a plethora of modifying factors (including sedimentological and biological criteria that encompass stratigraphic relations and vegetation type) influence the strength of bank materials (Hicken and Nanson, 1984), and electrochemical as well as mechanical properties affect the critical shear stress of cohesive sediment (Abdel-Rahmann, 1964; Grissinger, 1966; Kelly and Gularte, 1981; Kuijper *et al.*, 1989). Spatial discontinuities may also frustrate attempts to identify a unique resistance threshold for floodplains bordering any given river system (cf. Flaxman, 1963). Indeed, Fisk (1947) observed that fine-grained alluvial deposits tended to retard river migration in the lower Mississippi River valley. In addition, sediment concentration also affects the erosivity of the flow (Lane, 1953), and Huckleberry (1994) has shown that floodplain instability may be conditioned more by flow duration and volume than peak discharge. Thus, in common with other incipient motion problems, the resistance threshold for any particular floodplain will probably be defined by a range of values.

CONCLUSION

Two suites of overbank deposits were associated with the failure of artificial levees in the 70 km long study reach during the summer 1993 flood: (i) circumjacent sand deposits that are a component of the levee break complex that develops in the immediate vicinity of a break site; and (ii) suspended sediment deposition on the floodplain beyond the break site. Levee break complexes consist of an erosional, scoured and/or stripped zone, together with a circumjacent depositional zone. As exemplified by the Sny Island levee break complex, there was modest scour and stripping in the immediate vicinity of break sites within the study reach. The distinctive spur-and-furrow topography of the stripped zone often reflected the orientation of old plough furrows. Scour holes typically were not directly connected to adjacent chutes or to the main channel. In consequence, there was almost no sediment transport through the breaches and the circumjacent sand deposits present on the floodplain were derived primarily from levee material at the break site. The finite supply of sediment from the break site gave rise to the characteristic horseshoe morphology that the sand rim encircling the stripped zone exhibited. Except in the area immediately beyond the break site, where the impact of scour and stripping on the cohesive floodplain soils was most pronounced and towards which the main body of flow from the break site was probably directed, the amount of suspended sediment deposition on the floodplain was also modest. The aggregate pattern of suspended sediment deposition within the 20.2 km² Union Township Levee and Drainage District was resolved from a calibrated Landsat TM image, and validated by field survey. Estimated and measured sediment depths typically ranged from <1 to 4 mm, figures that were representative of the magnitude of overbank deposition on the floodplain throughout the study reach.

Cohesive floodplain soils probably limited scour at break sites, and the impression is that within the study reach the resistance of the riparian landscape to erosion generally exceeded the erosive power of this >100 year

flood. However, the disruption to the floodplain, evidenced at break sites within a 300 km long reach downriver from the confluence with the Missouri River, where the summer 1993 flood was a 10–50 year event, affords a somewhat different perspective. Within this reach the overall picture that emerged was one of greatly enhanced scour at break sites (although the scour typically did not link the break site to the main channel), and extensive sand deposition that was propagated by material derived from the scour hole. A reduction in the aggregate cohesion of the floodplain soils downriver from the confluence with the Missouri River probably precipitated the strikingly different response.

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